



# LIME TREATMENT IN HYDRAULIC STRUCTURES

DURABILITY AND OTHER ELEMENTS

Version 1 – 13<sup>th</sup> December 2023



# FRIANT KERN CHANNEL (CALIFORNIA, US)

## Construction details

- › 240 km irrigation channel
- › Built in 1946
- › Flow rate: 100 m<sup>3</sup>/s
- › Speed: 1,3 m/s
- ›  $I_p \sim 40$

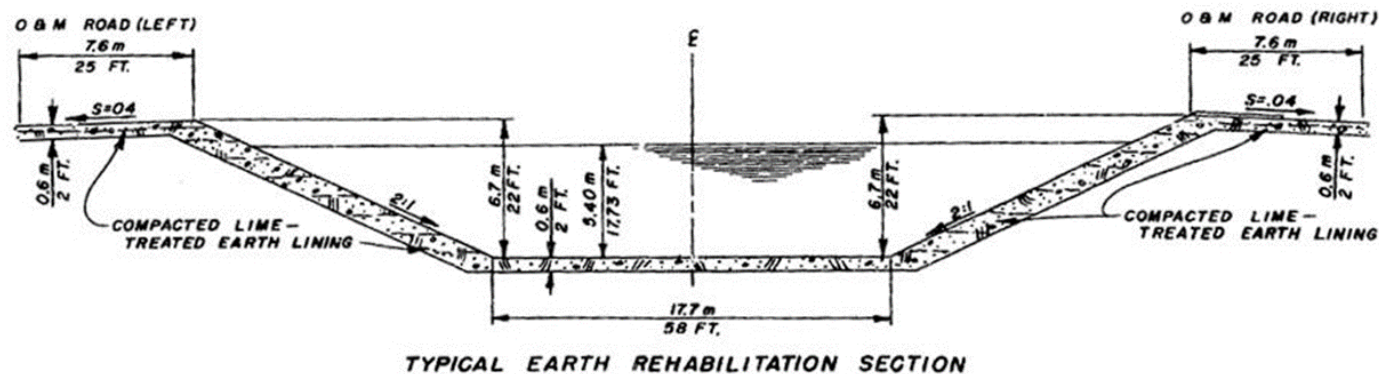
## Disorders since construction

- › Swelling clays ( $I_p \sim 40$ )
- › Slope slides
- › Failures of concrete cover



## Repair activities

- › 1972 – 1977: repair of 6,4 km banks
- › *Soil treated with 4% lime*



# FRIANT KERN CHANNEL (CALIFORNIA, US)

## Feedback after repairs

- › The treated area has since behaved satisfactorily
- › Not protected (protection initially planned with 20 cm concrete slab judged not useful)
- › **Not eroded** by flow
- › Requires **very little maintenance** (no erosion, no cleaning)

## Monitoring performed in 2020

Permanent effect of lime for over 40 years

- › Plastic characteristics of clays **permanently** altered
- › **Reduced shrinkage/swelling behaviour**
- › pH = 8,9 > 6,4 originally
- › **No lime leaching**
- › Presence of cementation products, calcite, available lime



[A.K. Howard, J.P. Bara (1976). **Lime stabilization on Friant-Kern Canal**. US Bureau of Reclamation, Report No. REC-ERC-76-20, Denver, CO (US), pp. 53-61]

[K. A. Gutschick (1985). **Canal lining stabilization proves successful**. Pit & Quarry, pp. 58-60]

[G. Herrier et al. (2012). **Principles and properties of soils treated by lime for hydraulic earthen structures**. Proceedings of the 3<sup>rd</sup> International Seminar on Earthworks in Europe, Berlin (D)]

[P. Akula et al. (2020). **Evaluating the Long-Term Durability of Lime Treatment in Hydraulic Structures: Case Study on the Friant-Kern Canal**. Transportation Research Record 0361198120919404]

[Belgian Research Road Center. Code of good practice R103 (2021). **Soil treatment with lime – European expérences for soil improvement and soil stabilisation. State of the art**. Brussels (BE), pp. 88-93]

# MISSISSIPPI DIKES (US)

## Construction details

- › 350 km network of dikes along the Mississippi river
- › Built between 1940 and 1950
- › Local materials, incl. very plastic clays ( $I_p > 50$ )
- › Height: 7 to 8 m
- › Slope: 3/1

## Disorders since construction

- › Cracks in the body during dry periods (1 to 2 m deep)
- › Water infiltration during wet periods through cracks
- › Slope too steep for low residual resistance of very plastic swelling clays
- › Surface slidings (tens of m) or even failure



## Repair activities

- › Until 1974 - excavation and replacement with better quality soil and softening of slopes
- › After 1974 – absence of replacement materials:
  - **Lime treated soil**
  - Renovation of 142 embankments
  - Reinforcement of 45 km (early 2010s)

[Belgian Research Road Center. Code of good practice R103 (2021). Soil treatment with lime – European expériences for soil improvement and soil stabilisation. State of the art. Brussels (BE), pp. 88-93]

[G. Herrieret al. (2012). Principles and properties of soils treated by lime for hydraulic earthen structures. Proceedings of the 3<sup>rd</sup> International Seminar on Earthworks in Europe, Berlin (D)]

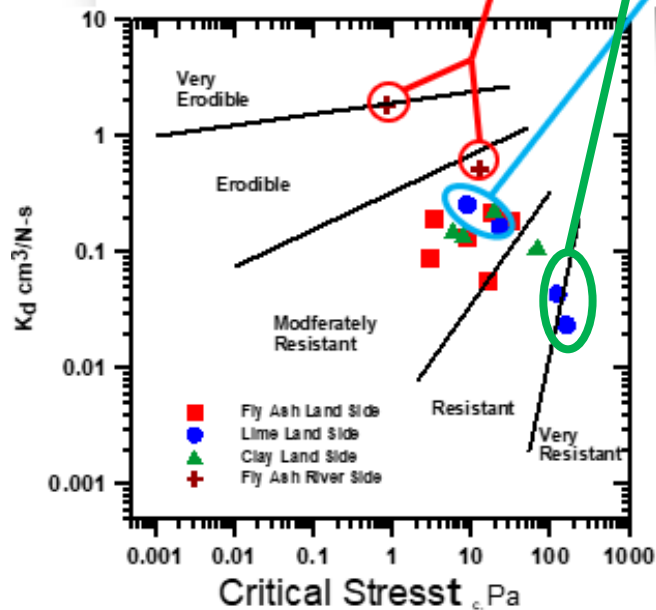
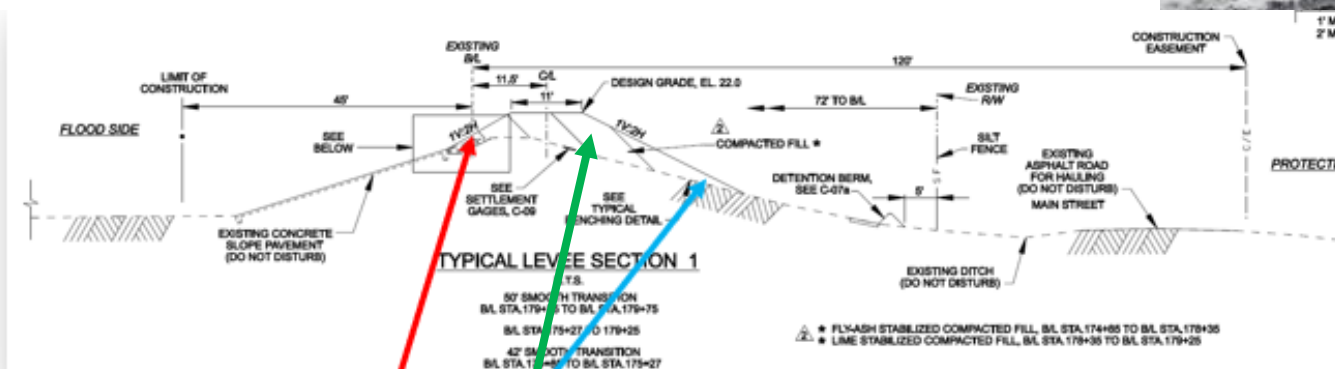
[P. Forsythe (1977). Experiences in identification and treatment of dispersive clays in Mississippi dams. In J.L. Sherard & R.S. Decker (Eds.), ASTM Special Technical Publication: STP 623. Dispersive clays, related piping, and erosion in geotechnical projects. (pp. 135-155). ASTM International. <https://doi.org/10.1520/STP26985S>]

[R.L. Fleming, G.L. Sills, E.S. Stewart (1992). Lime stabilization of levee slope. 2<sup>nd</sup> interagency symposium on stabilization of soils and other materials, Metairie, LA (US), pp. 79-87]

# MISSISSIPPI DIKES (US)

## Reinforcement of 45 km in (early 2010s)

- › Plaquemine Parish dike
- › *Treatment with 8% lime*
- › In-situ JET tests



**Raised geometry:  
compaction efficiency  
issues**

Ref: USACE – Vicksburg



# VARIOUS CASES (CZECH REPUBLIC)

## Dike of Chobot lake (2002)

- › Breach after 2002 floods
- › Reconstruction with *lime treatment*



## Dike of Hvezda lake (2004)

- › Damages after 2002 floods
- › Difficulties of reconstruction due to very wet soils
- › Reconstruction with *lime treatment*



## Protective dike in Hradec Kralové (2005)



# VARIOUS CASES (FRANCE)

## Borre Pradelles dike (2013)

- › Expansion of the Bourre flood
- › Local material (A2) *treated with 2-3% lime*

## Small protective dikes in Normandy/Northern France

- › *Lime* used in construction site for a basin
- › Very old use in the region where the soils are easily treated
- › Good knowledge of public works companies
- › Protection plans coordinated by water agencies and local administrative authorities
  - › ASYBA CEREMA Rouen Seminar in March 2014

## Reparation of the dike of the Maurianges pond (2023)

- › Numerous leaks and loss of sealing at the level of the drain structure
- › Changing the drain structure
- › Earthworks and *treatment* of materials with *lime* in embankment



# EDIT: ROUEN DEMONSTRATOR (FRANCE)

## Construction details

- › Silty Soil ( $I_p = 11$ )
- › **2,5% lime treatment**
- › Height: 2m
- › Slope: 3/2
- › Mobile treatment facility

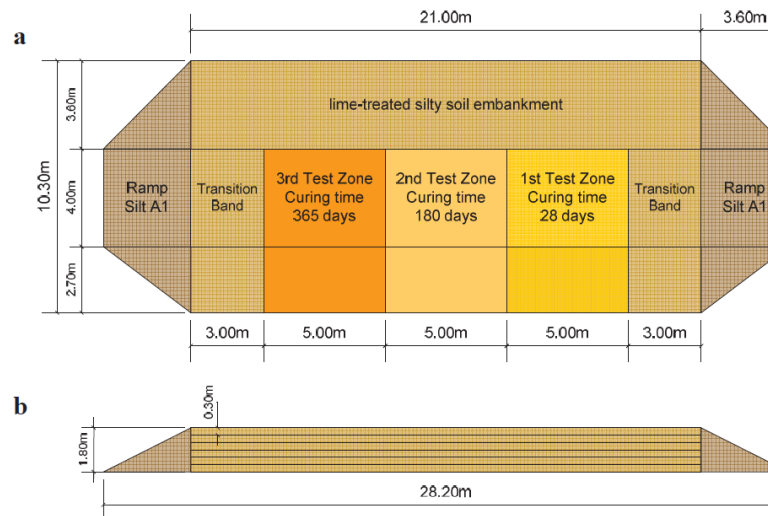
## Testing & Monitoring activities

### After 1 year

- › Permeability
- › Coring - mechanical performance
- › Internal and external erosion
- › Vegetation

### After 7/8 years

- › Physico-chemical analysis
- › Sampling of blocks - mechanical performance
- › Microstructural analysis





# EDIT: ROUEN DEMONSTRATOR (FRANCE)

## Main feedback

### After 1 year

#### › Low *permeability*

Nasberg test and triaxial cells on core samples:  $10^{-9}$  -  $10^{-10}$  m/s

#### › Increased *erosion resistance*

MoJET and HET tests: x7 to x10 untreated soil

#### › Resistance to deep *root penetration*

#### › Increased *mechanical performance*

Bearing capacity x5 to x30 after 1 month

Cohesion x7 to x12 untreated soil

### After 7 years

#### › Stable high *mechanical resistance*

$R_c \sim 3$  MPa (average, but heterogeneities)

#### › *Stability* of pozzolanic reaction products

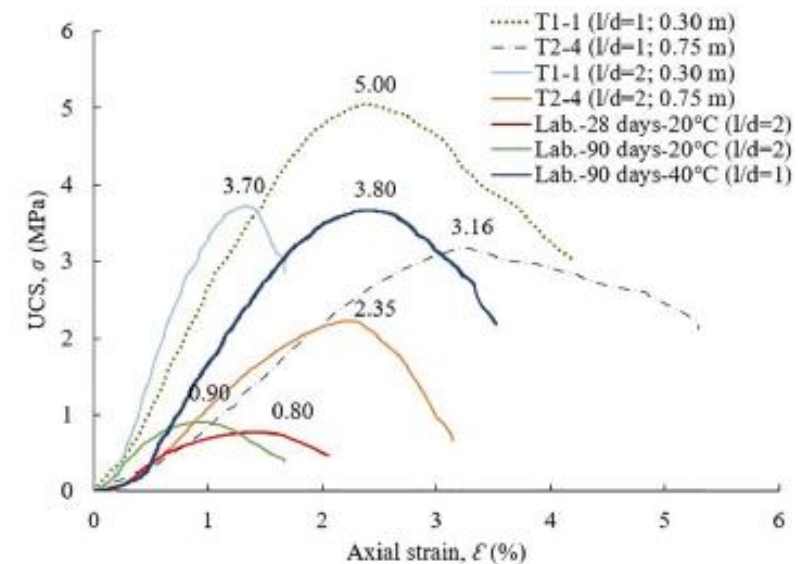
#### › Improvement of *water retention capacity* of the embankment

#### › *High pH* maintained in the backfill (>11)

Loss of lime effect on a small surface thickness ( $\sim 10$  cm)



Type d'essai		Unité	DIGUE EN LIMON NON TRAITÉ		DIGUE EN LIMON TRAITÉ À LA CHAUX	
					28 jours	180 jours
HET (Hole Erosion Test, érosion de trou)	Contrainte critique	kPa	179	→ 1000 (non atteinte)	± 800 (1 <sup>er</sup> essai) → 1000 (non atteinte, second essai)	
	Coefficient d'érosion $k_{er}$	s/m	$9,8 \cdot 10^{-5}$	Non mesurable	-	
	Indice d'érosion $(=-\log k_{er})$	-	4,04	Non mesurable	-	



[I. Charles et al. (2012). An experimental full-scale hydraulic earthen structure in lime treated soil. Proceedings of the Sixth International Conference on Scour and Erosion, Paris (F), Aug. 2012: 181–188]

[G. Herrier et al. (2012). Principles and properties of soils treated by lime for hydraulic earthen structures. Proceedings of the 3<sup>rd</sup> International Seminar on Earthworks in Europe, Berlin (D), Mar. 2012]

[I. Charles et al. (2015). A real scale experimental dike in lime-treated soil: Evaluation of the methodology, mechanical and hydraulic performance. Scour and Erosion – Cheng, Draper & An (Eds), Taylor & Francis Group, London (UK), 978-1-138-02732-9]

[L. Makki-Szymkiewicz et al. (2015). Evolution of the properties of lime-treated silty soil in a small experimental embankment. Engineering Geology, 191, pp. 8-22]

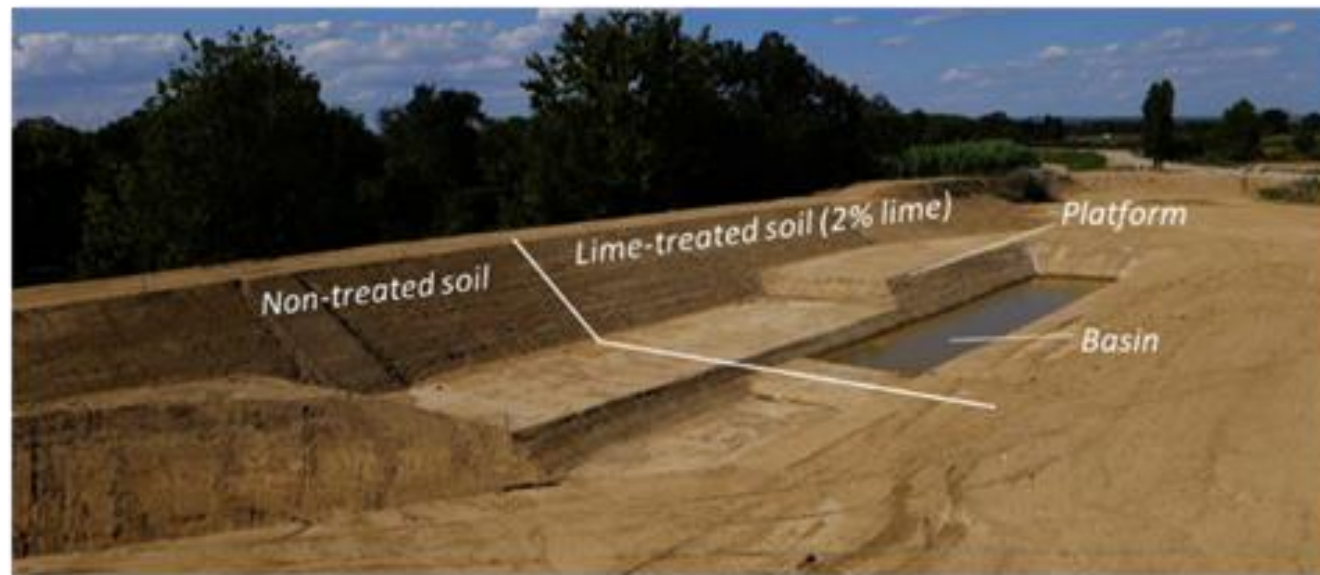
# DIGUE ELITE: VIDOURLE DEMONSTRATOR (FRANCE)

## Construction details

- › Silty soil ( $I_p = 5$ )
- › **2% lime treatment**
- › Height: 3,5m
- › Slope: 3/2
- › Mobile treatment facility

## Testing & Monitoring activities

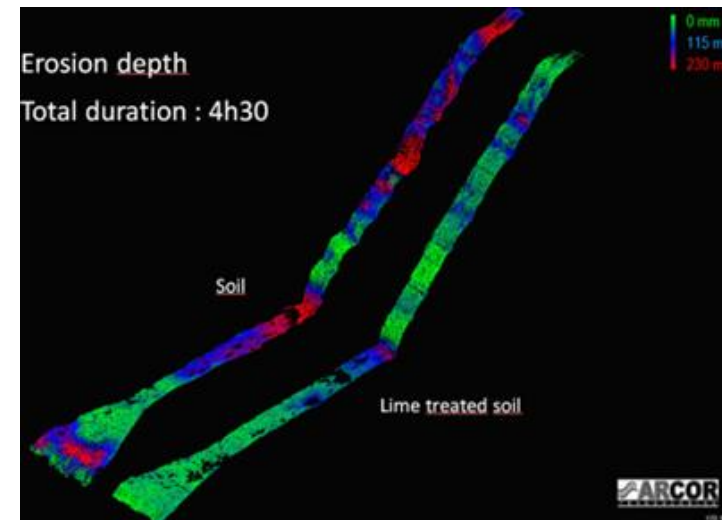
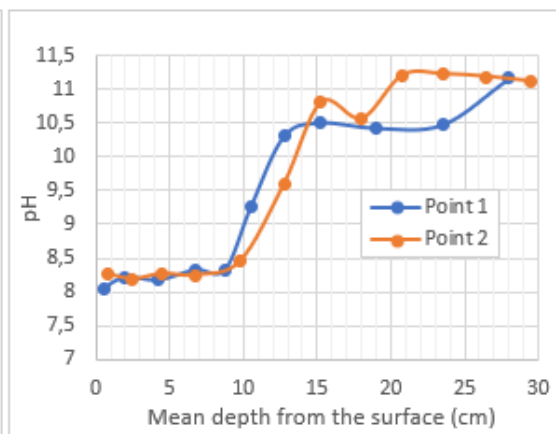
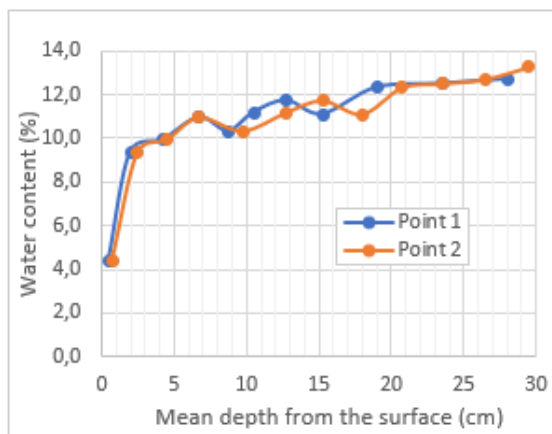
- › Lab feasibility tests
- › After construction
  - › External erosion JET (1 – 6 years)
  - › Overflowing tests (9 – 22 months)
  - › Mechanical performance (7 years)
  - › Physico-chemical analysis (6 years)



# DIGUE ELITE: VIDOURLE DEMONSTRATOR (FRANCE)

## Main feedback

- › **Low permeability:**  $10^{-8}$  to  $10^{-10}$  m/s after 6 years from the construction
- › Increased resistance to **internal and external erosion**  
 Overflowing test & JET: x2 to x100 more resistant after 6 years ( $50 < \tau_c < 2000$  Pa)  
 HET test in-situ sampling: no erosion ( $\tau_c > 500$  Pa - device measurement threshold)
- › Increase and maintenance of **mechanical resistance** (some heterogeneities)  
 0,2 MPa natural soil | 1,2 MPa soil reconstituted in lab (1 year) |  $0,8 < R_c < 2,4$  MPa (6 years)  
 Cohesion x12 – x20 compared to natural soil
- › **Superficial cracks** rapid drying/suction phenomena (high temperature during construction)
- › Thickness of **environmental degradation:** 10-20 cm



[N. Nerinx et al. (2016). Impact of lime treated soils performance on design of earthfill dikes. FLOODrisk 2016 - 3<sup>rd</sup> European Conference on Flood Risk Management, E3S Web of Conferences 7, 14004]

[N. Nerinx et al. (2018). The DigueELITE project: lessons learned and impact on the design of levees with lime treated soils. Hydropower and Dams, vol. 25, issue 6]

[G. Herrier et al. (2018). Erosion resistant dikes thanks to soil treatment with lime. 3<sup>rd</sup> International Conference on Protection against Overtopping, 6-8 June 2018, Grange-over-Sands (UK)]

[S. Bonelli et al. (2018). Quantifying the erosion resistance of dikes with the overflowing simulator. 3<sup>rd</sup> International Conference on Protection against Overtopping, 6-8 June 2018, Grange-over-Sands (UK)]

[F. Bertola et al. (2024). Evaluation of the geomechanical properties of lime-treated silt samples extracted from an experimental levee 6 years after the construction. XVIII ECSMGE 24, Lisbon (P) - will be presented 26-30 August 2024]

# SALIN DE GIRAUD DEMONSTRATOR (FRANCE)

## Construction details

- › Mix of sandy - silty soil + clay (average  $I_p = 9$ )
- › **2% lime treatment**
- › Height: 2m
- › Slope: 2,5/1
- › Treatment on the platform
- › Saline environment

## Testing & Monitoring activities

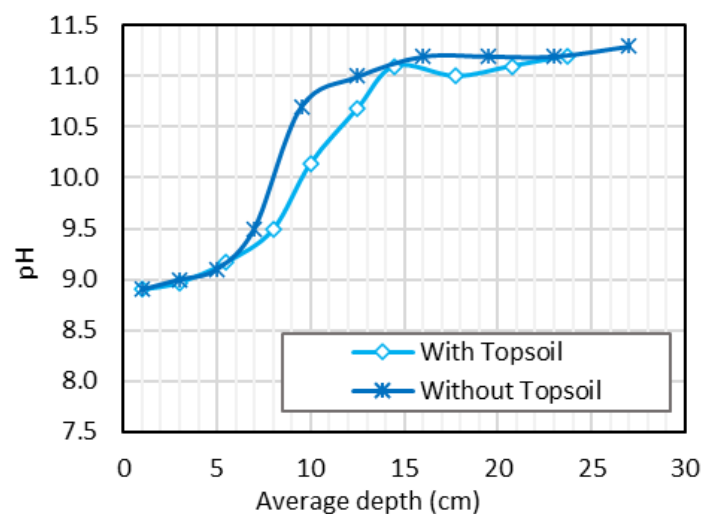
- › Lab feasibility tests
- › After construction
  - › Overflowing tests (6 months)
  - › Internal/external erosion HET/JET on cores (1 year)
  - › JET tests (5 years)
  - › Mechanical tests on cores (5 years)
  - › Physico-chemical analysis (5 years)



# SALIN DE GIRAUD DEMONSTRATOR (FRANCE)

## Main feedback

- › **Low permeability:**  $10^{-9}$  to  $10^{-10}$  m/s after 5 years from the construction
- › Very good resistance to **external erosion** (overflowing tests)
  - Little to no erosion vs breakdown of untreated soil (2h of stress under  $q = 400$  l/s/m,  $h = 35$  cm)
- › High resistance to **internal erosion**
  - HET tests after 5 years ( $\tau_c = 450$  Pa)
  - JET tests on samples taken in situ after 1 year ( $\tau_c = 350$  Pa)
  - JET tests on surface after 5 years ( $100 < \tau_c < 150$  Pa) to be compared with values on material taken in situ (in progress)
- › Mechanical characterization ongoing (5 years)



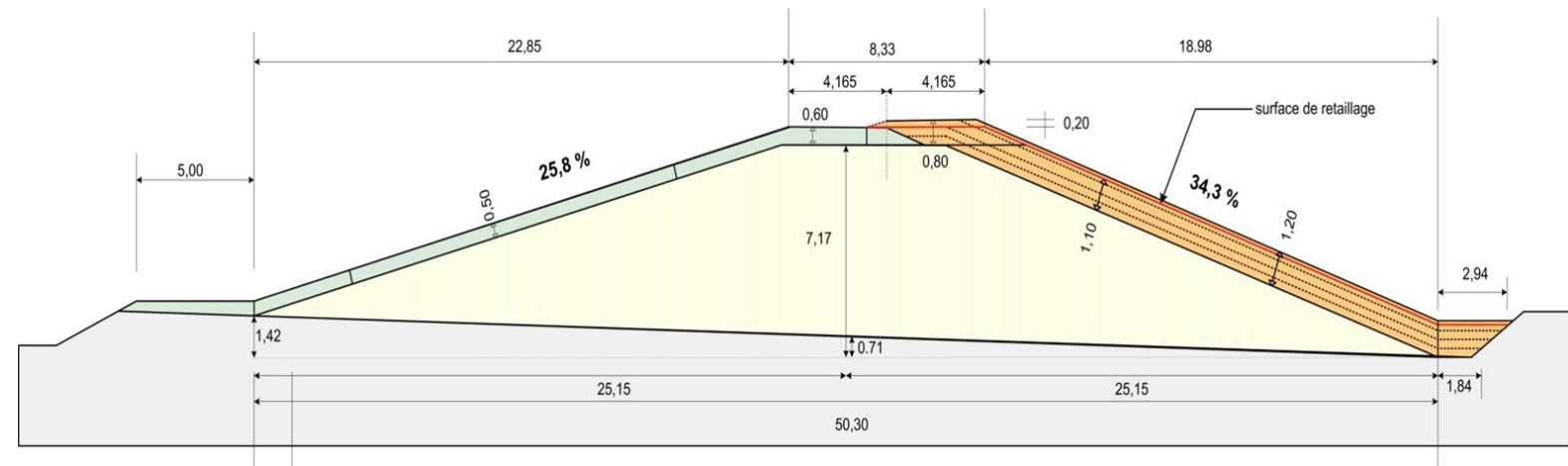
# VLASSEN BROEK DEMONSTRATOR (BELGIUM)

## Construction details

- › Silty Soil ( $I_p = 8-10$ ) → **2% lime treatment**
- › Sandy Soil ( $I_p = 3$ ) → **3% formulated lime treatment**
- › Protective shell
- › Height: 6,5m
- › Slope: 3/1
- › Treatment on the platform
- › Compaction in the direction of the slope

## Testing & Monitoring activities

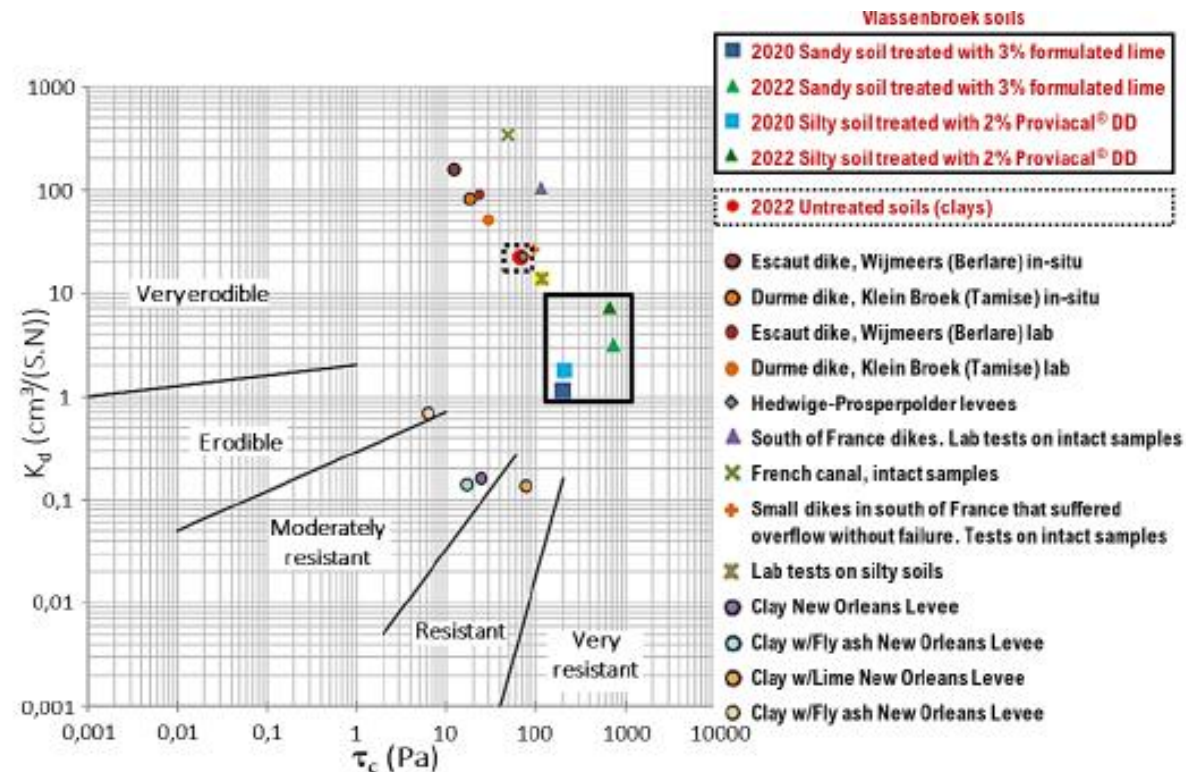
- › Lab feasibility tests
- › After construction
  - › Permeability
  - › Vegetation growth
  - › Mechanical performance
  - › External erosion (JET)
  - › Overflowing tests planned for 2024/2025



# VLASSEN BROEK DEMONSTRATOR (BELGIUM)

## Main feedback

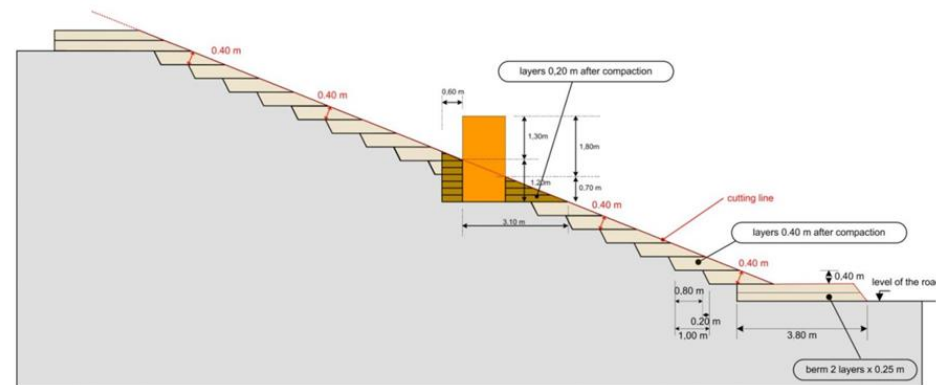
- › Increase **external erosion** through JET (1 year to 3 years)  
 $160 < \tau_c < 250$  Pa to  $280 < \tau_c < 1000$  Pa
- › Gradual improvement in **vegetation growth**
- › Monitoring of **permeability** by infiltration tests using tubes or double rings on in-situ samples  
 Average permeability  $\sim 10^{-7}$  m/s
- › Evolution of **mechanical resistance** monitored with a light penetrometer  
 Too high resistance (in the first 20-40 cm) after 1 year  
 Onsite coring planned for 2024-2025
- › Overflowing tests planned for 2024-2025



# HEDWIGEPOLDER DEMONSTRATOR (THE NETHERLANDS)

## Construction details

- › Clayey Soils ( $I_p = 20$  to 45)
- › **4% - 5% lime treatment**
- › Protective shell
- › Height: 8m
- › Slope: 2,5/1
- › Treatment on the platform
- › Staircase compaction
- › Presence of obstacles on the slope

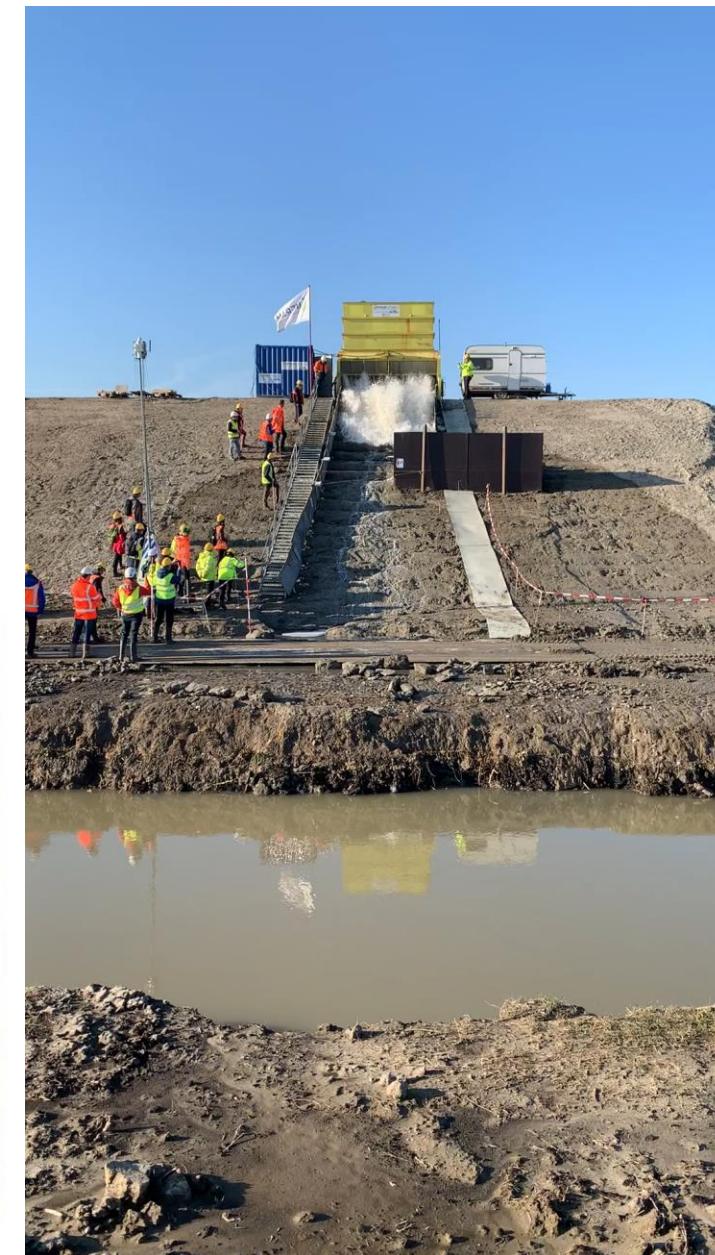
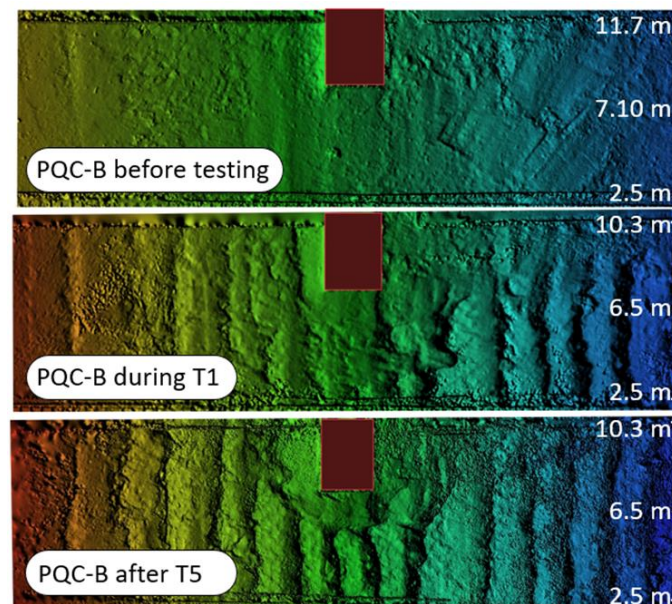


## Testing & Short term monitoring activities

- › Lab feasibility tests
- › After construction: overtopping tests after 3 months

## Main feedback

- › Good resistance to **external erosion**
- › Improved **availability** of usable materials
- › Increased **resistance to erosion around obstacles**





# QUESTIONS?

Please contact:  
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[federica.bertola@lhoist.com](mailto:federica.bertola@lhoist.com)

