

**KU LEUVEN**



# A New Sediment Transport Model for Western Scheldt

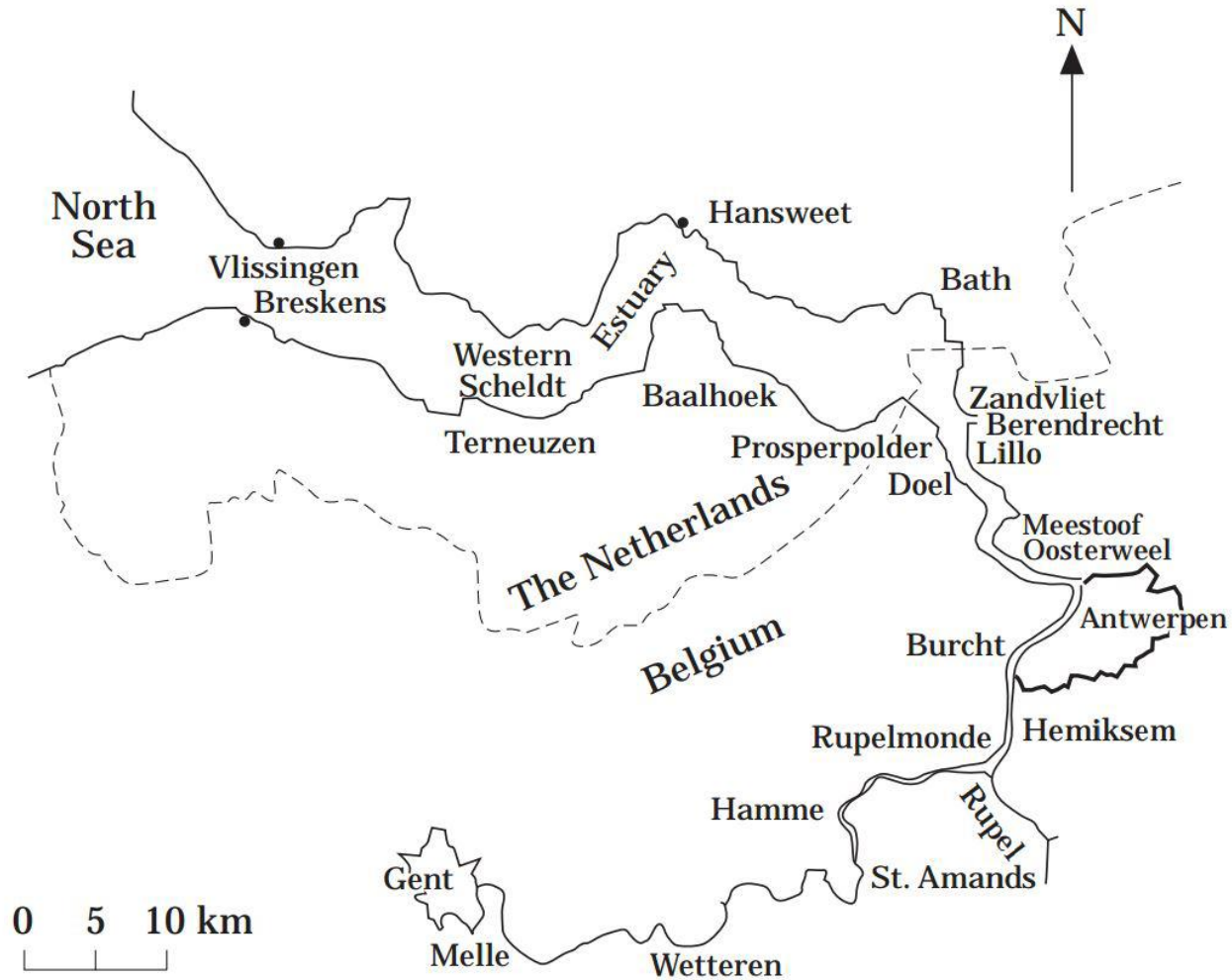
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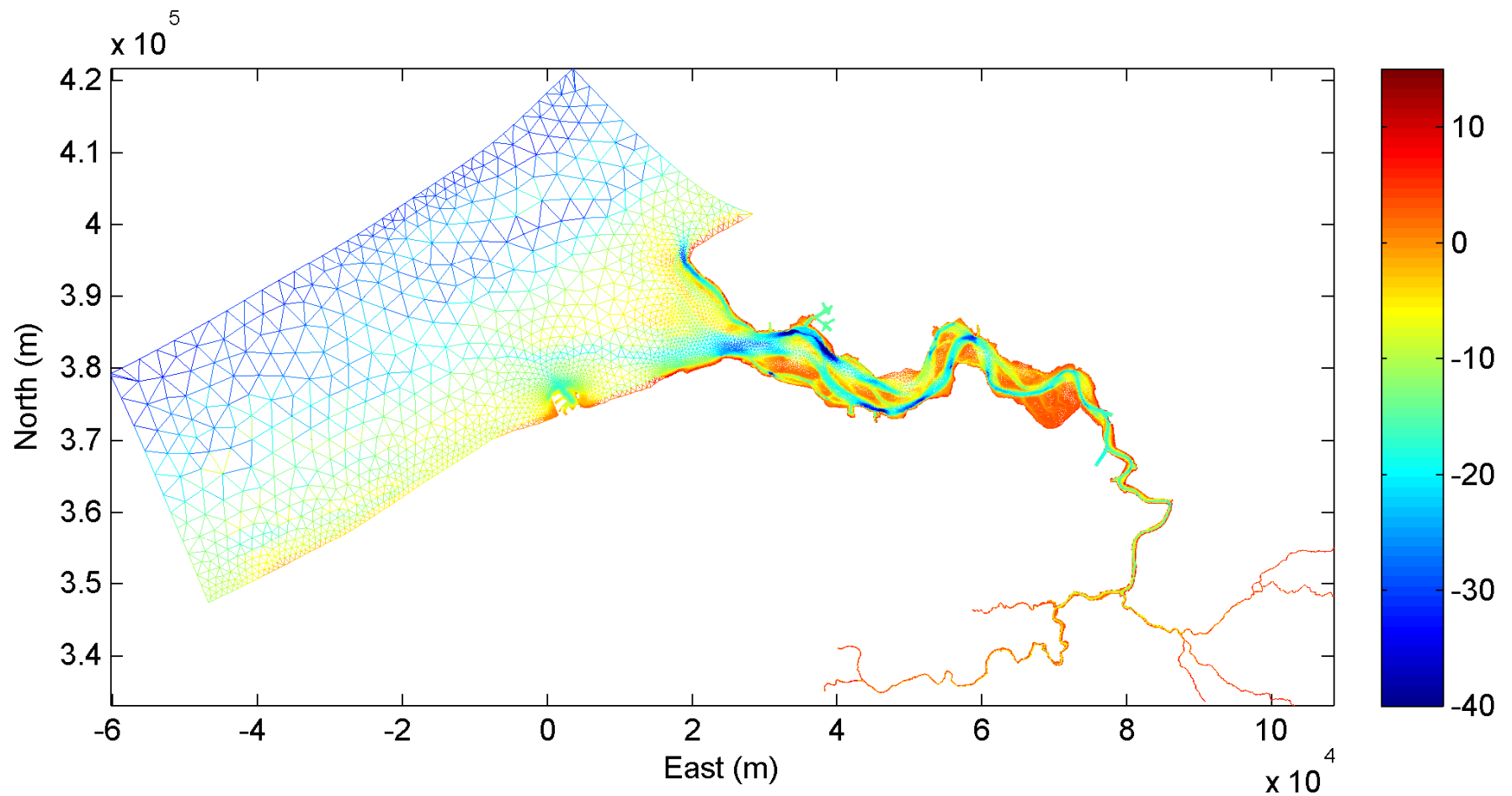


# The Scheldt Estuary



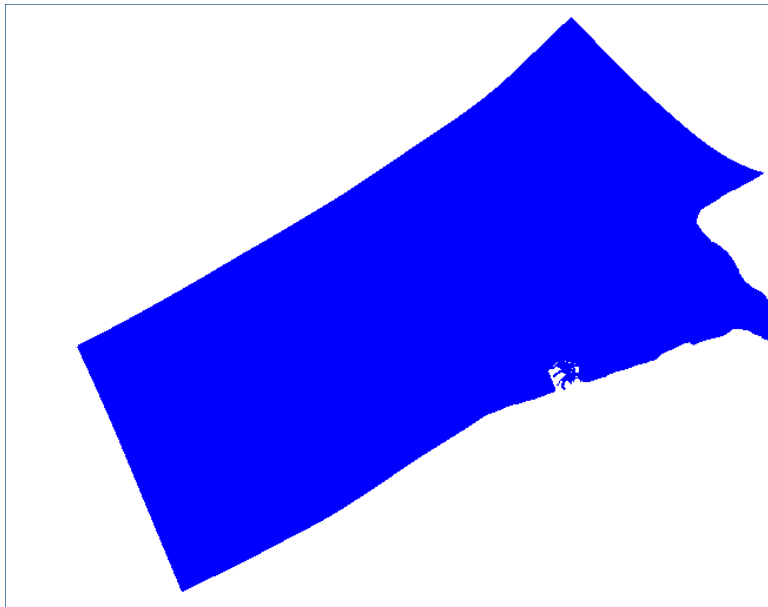
(from Fettweis et al., 1998)

# Hydrodynamic Model



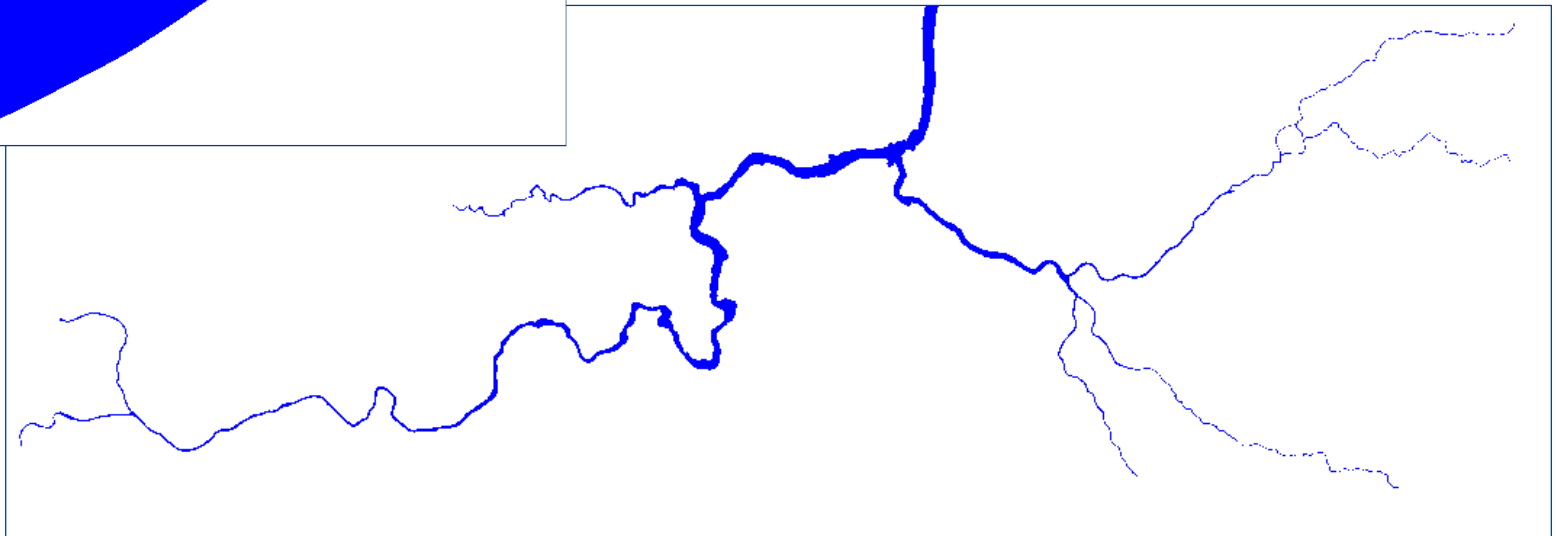
(Mesh and bathymetry of the Scheldt model)

# Hydrodynamic Model



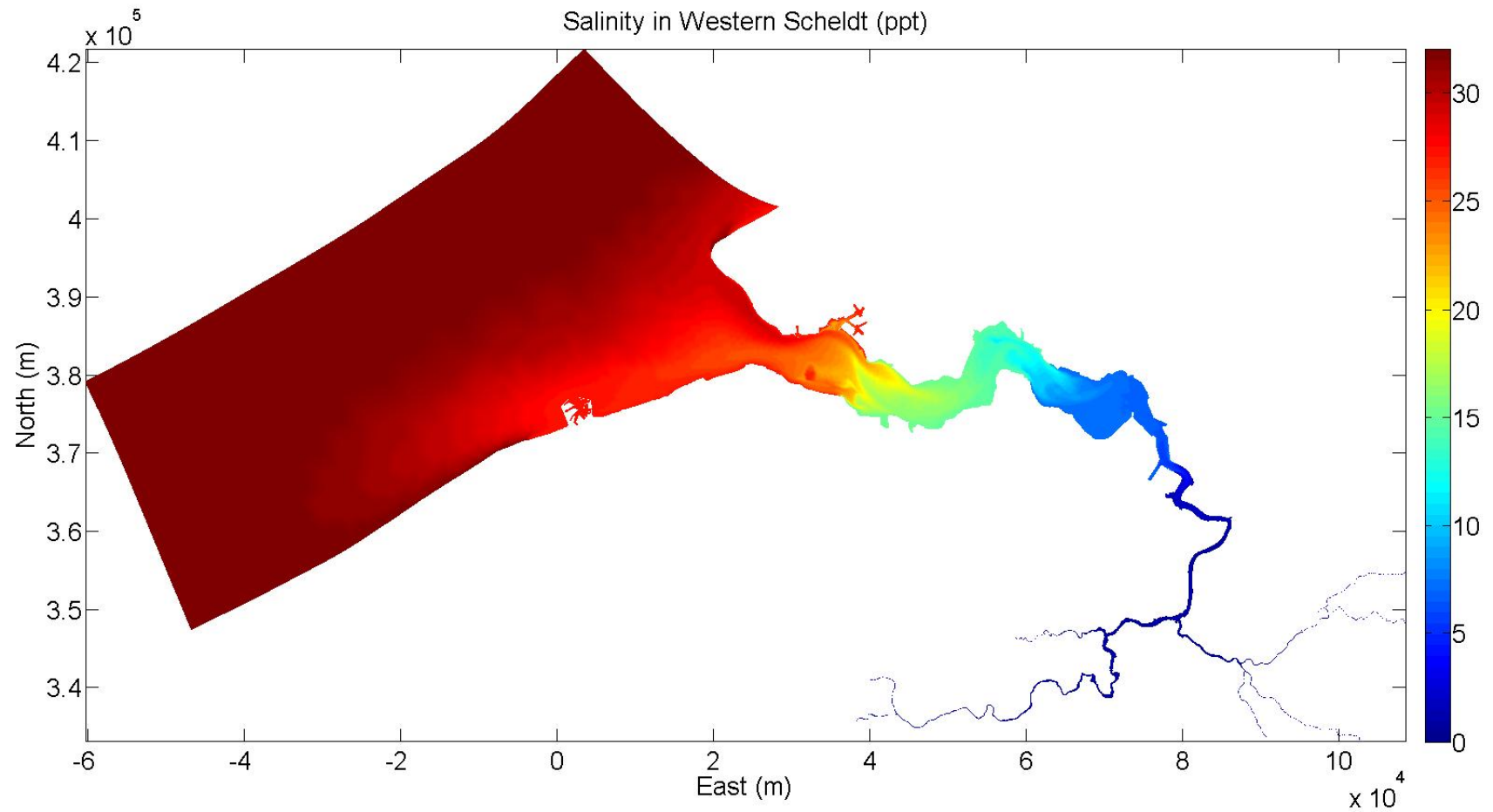
Upstream: constant fresh water inflow from tributaries

Downstream: imposed tidal elevations and salinity data



(Liquid boundaries)

# Hydrodynamic Model



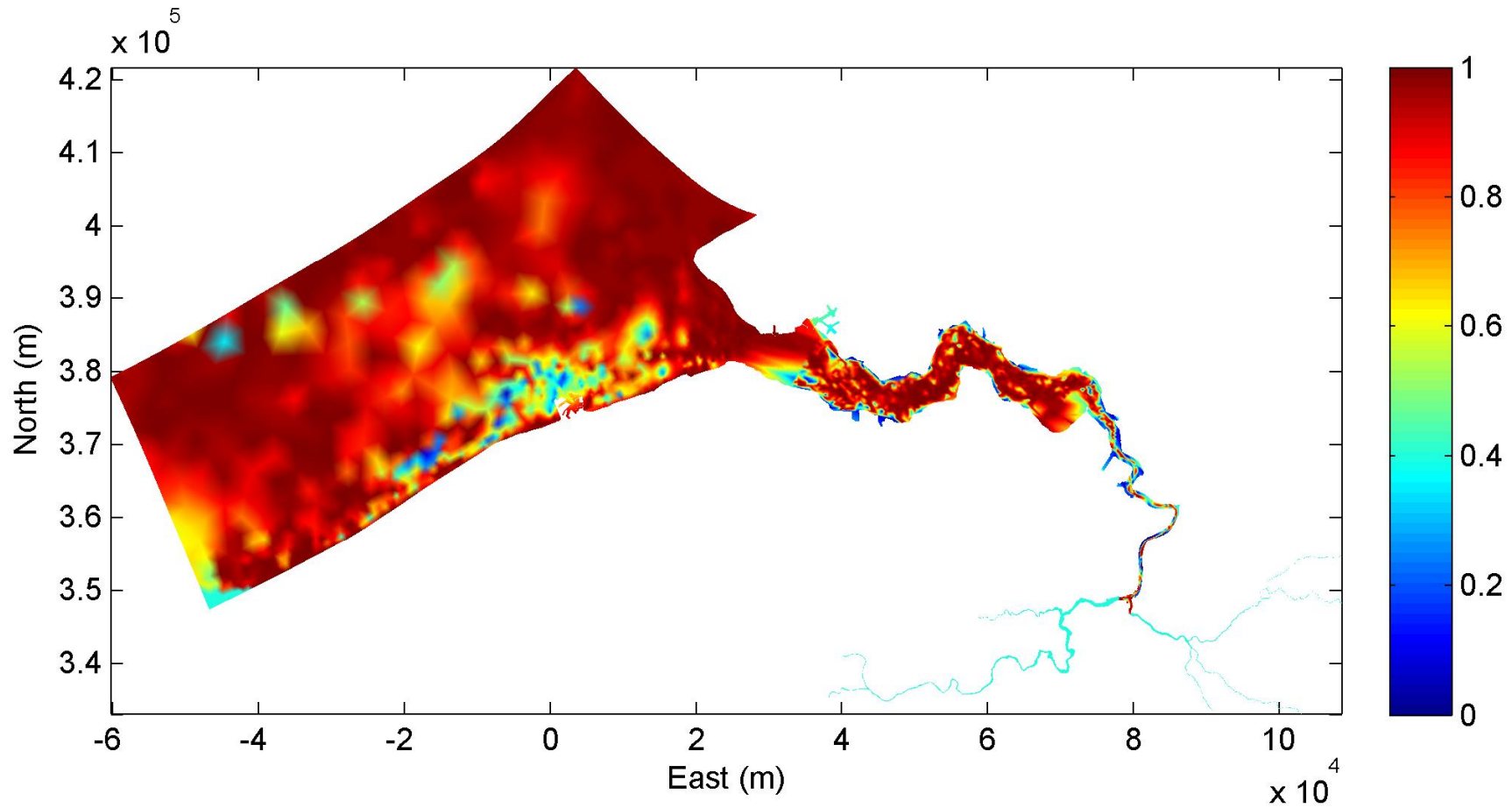
(Initial salinity)

# Hydrodynamic Model

## Other settings

- Law of bottom friction : Chézy formula
- Turbulence model : k-epsilon model
- Treatment of tidal flats :  
equations solved everywhere with correction on tidal flats
- Coriolis force considered
- Coupled with SISYPHE

# Sediment Transport Model

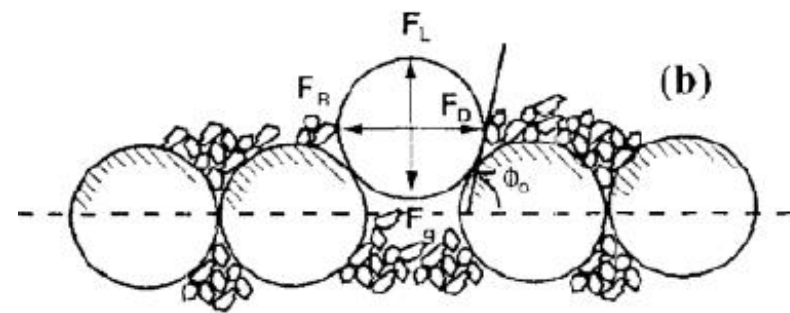
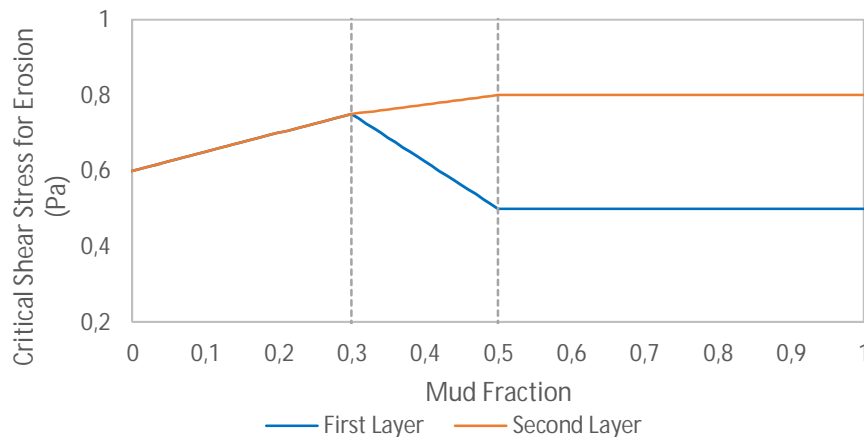


(mixed-sediment: sand fraction distribution map)

# Sediment Transport Model

Sediment properties (two-layer bed model)

Type of Sediment	Diameter	Density	Settling Velocity	Critical Shear Stress for Erosion (first layer)	Critical Shear Stress for Erosion (second layer)
Non-cohesive (Sand)	300 $\mu\text{m}$	2560 $\text{kg/m}^3$	1.0E-02 m/s	0.6 Pa	0.6 Pa
Cohesive (Mud)	60 $\mu\text{m}$	1600 $\text{kg/m}^3$	1.0E-03 m/s	0.5 Pa	0.8 Pa



(Waeles, 2005)



# Sediment Transport Model

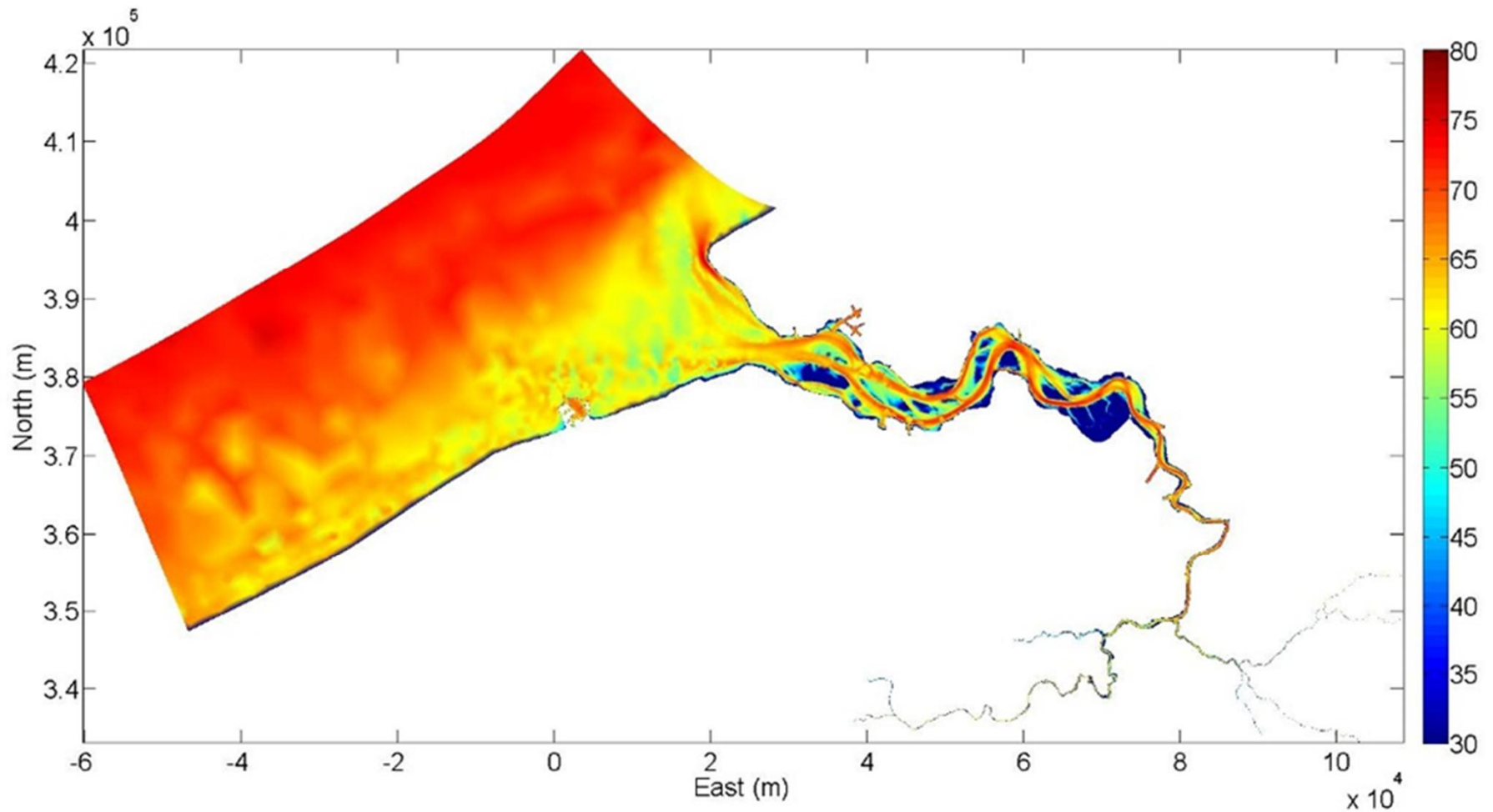
- The new bottom friction Law based on GML theory
  - General case (*laminar – transient – turbulent*):

$$u_*^2 = f_A u_{*turb}^2 + u_{*lam}^2$$

$$u_*^2 = f_A \left( \frac{\kappa U}{\ln(h/z_0) - 1 + z_0/h} \right)^2 + \left( \sqrt{\left( z_0 \frac{U}{h} \right)^2 + 2 \frac{U}{h} v} - z_0 \frac{U}{h} \right)^2$$

- with:  $z_0 = \frac{k_s}{30} + \beta \phi h$

# Sediment Transport Model

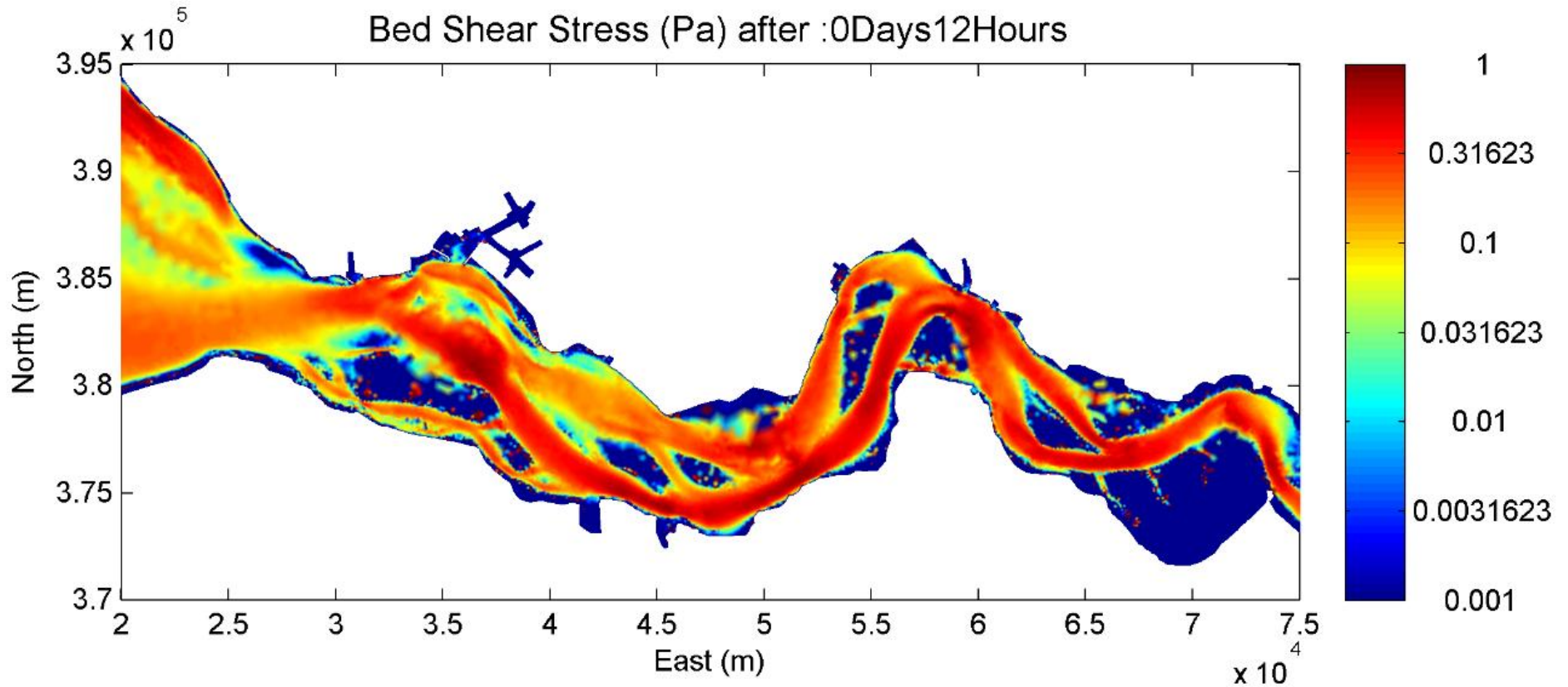


(Chézy coefficient obtained from the new bottom friction law)

# Sediment Transport Model

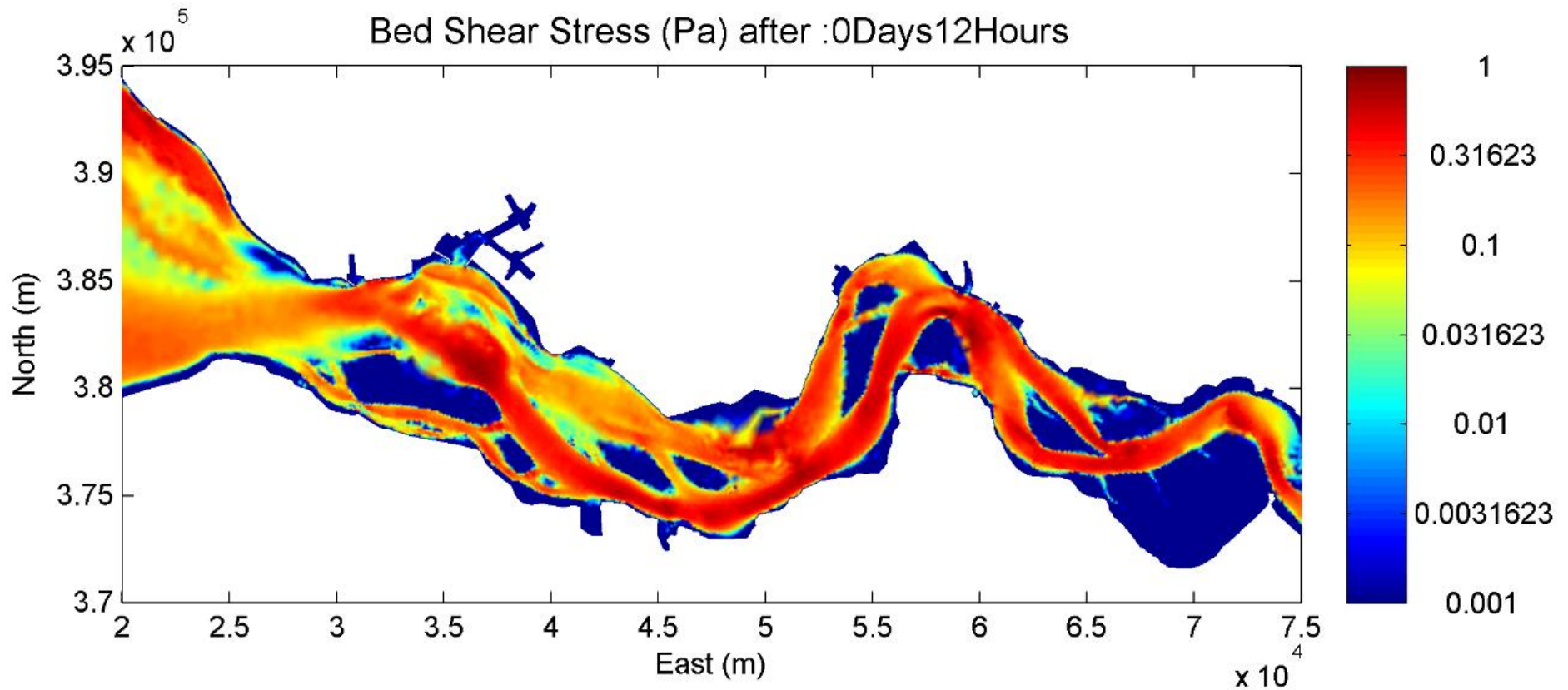
- Advantages of the new bottom friction law
  - More physically based, more accurate
  - Allows reduction of inundation threshold to roughness height (1mm in current model)
  - Improves bed shear stress computation in very shallow areas (intertidal flats)

# Sediment Transport Model



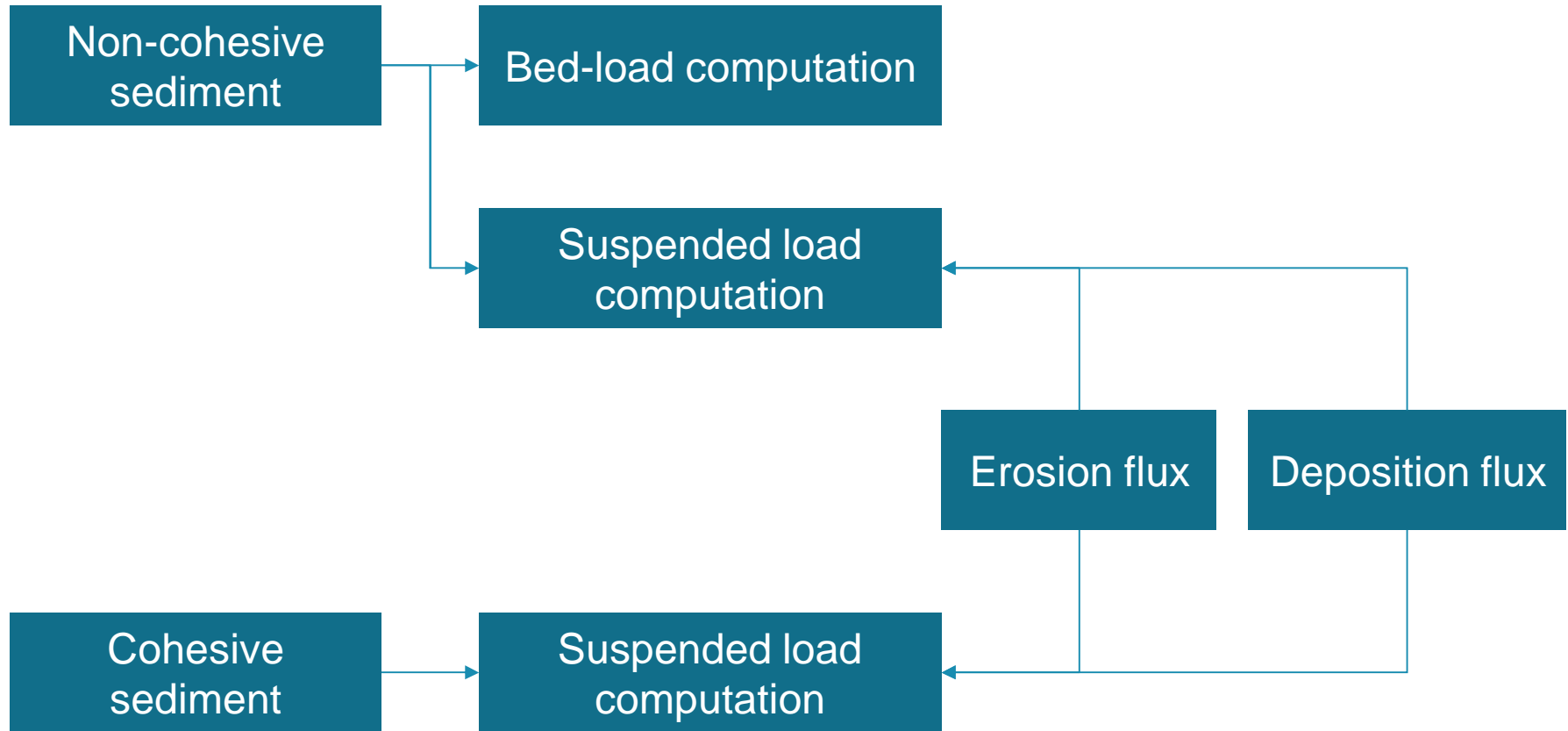
In dry areas, velocity  $U$  tends to be zero  $\rightarrow u_* = \sqrt{C_D / 2U}$

# Sediment Transport Model



In dry areas,  $u^*$  is limited to zero by the new friction law

# Sediment Transport Model



# Sediment Transport Model

- **Bed-load:**  
the formula from Van Rijn (1984)  $q^* = \frac{0.053}{d^{*0.3}} \left( \frac{\tau^*}{\tau_{crit}^*} - 1 \right)^{2.1}$
- **Suspended load:** depth-averaged advection-diffusion equation

$$\frac{\partial hC}{\partial t} + \frac{\partial (hUC)}{\partial x} + \frac{\partial (hVC)}{\partial y} = \frac{\partial}{\partial x} \left( h\varepsilon_s \frac{\partial C}{\partial x} \right) + \frac{\partial}{\partial y} \left( h\varepsilon_s \frac{\partial C}{\partial y} \right) + E - D$$

- **Erosion: Waeles (2005)**

Non-cohesive regime  $f_m < 30\%$

$$\begin{aligned} E_s &= (1 - f_m) \cdot E_{0s} \cdot T^a \\ E_m &= f_m \cdot E_{0s} \cdot T^a \end{aligned}$$

linear  
interpolation

Cohesive regime  $f_m > 50\%$

$$\begin{aligned} E_s &= (1 - f_m) \cdot E_{0m} \cdot T \\ E_m &= f_m \cdot E_{0m} \cdot T \end{aligned}$$

# Sediment Transport Model

- The new deposition criterion  
Critical shear stress for deposition:

$$\tau_{cd,s} = \frac{(1 - \rho_w / \rho_s) gh w_s C_s}{Rf_L U} \quad \tau_{cd,m} = \frac{(1 - \rho_w / \rho_m) gh w_m C_m}{Rf_L U}$$

## Deposition probabilities

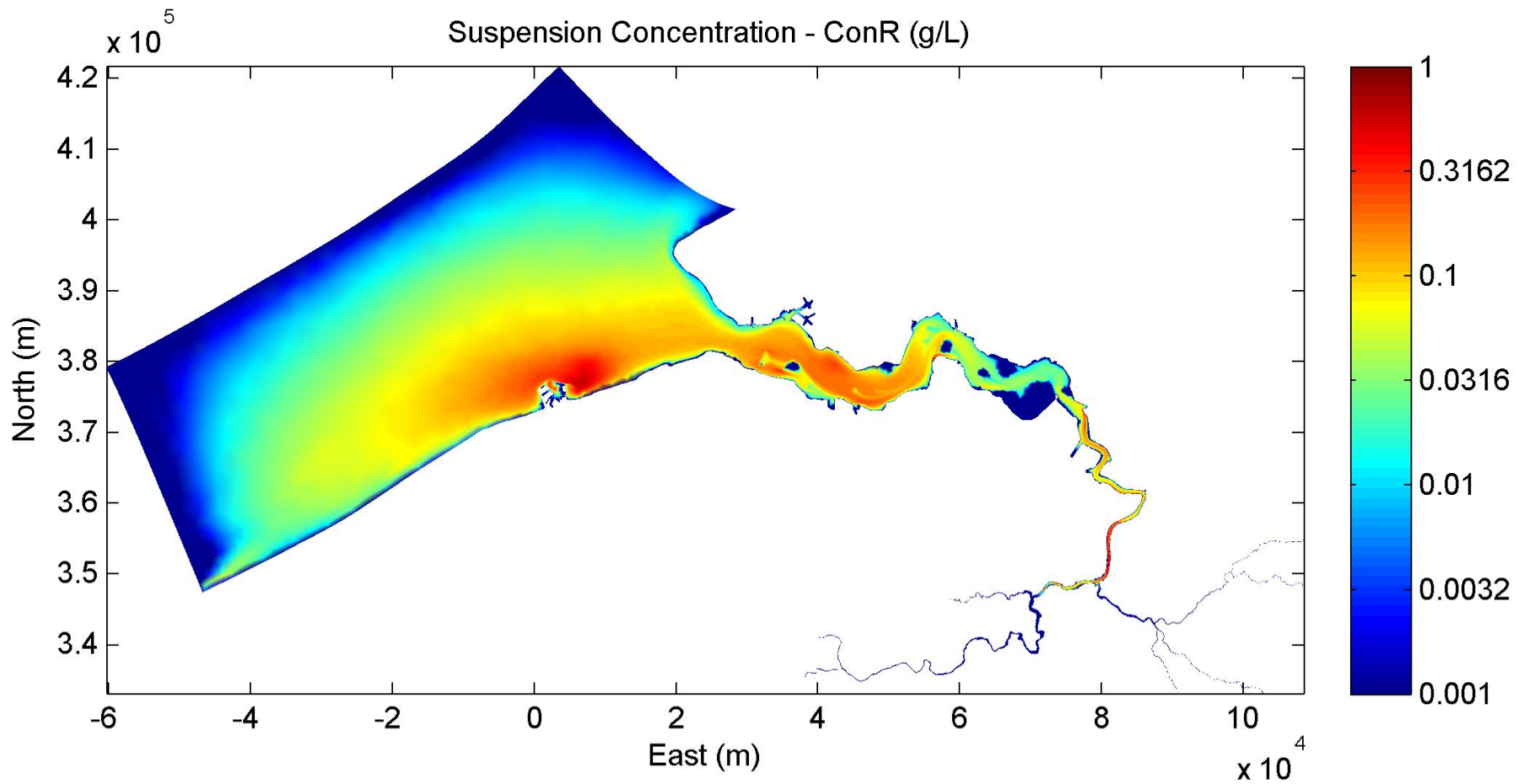
$$p_s = \max \left[ 1 - \frac{C_s}{C_s + C_m} \frac{\tau}{\tau_{cd,s}}, 0 \right] \quad p_m = \max \left[ 1 - \frac{C_m}{C_s + C_m} \frac{\tau}{\tau_{cd,m}}, 0 \right]$$

- Krone's deposition law

$$D_s = (w_s C_s) \cdot p_s \quad D_m = (w_m C_m) \cdot p_m$$

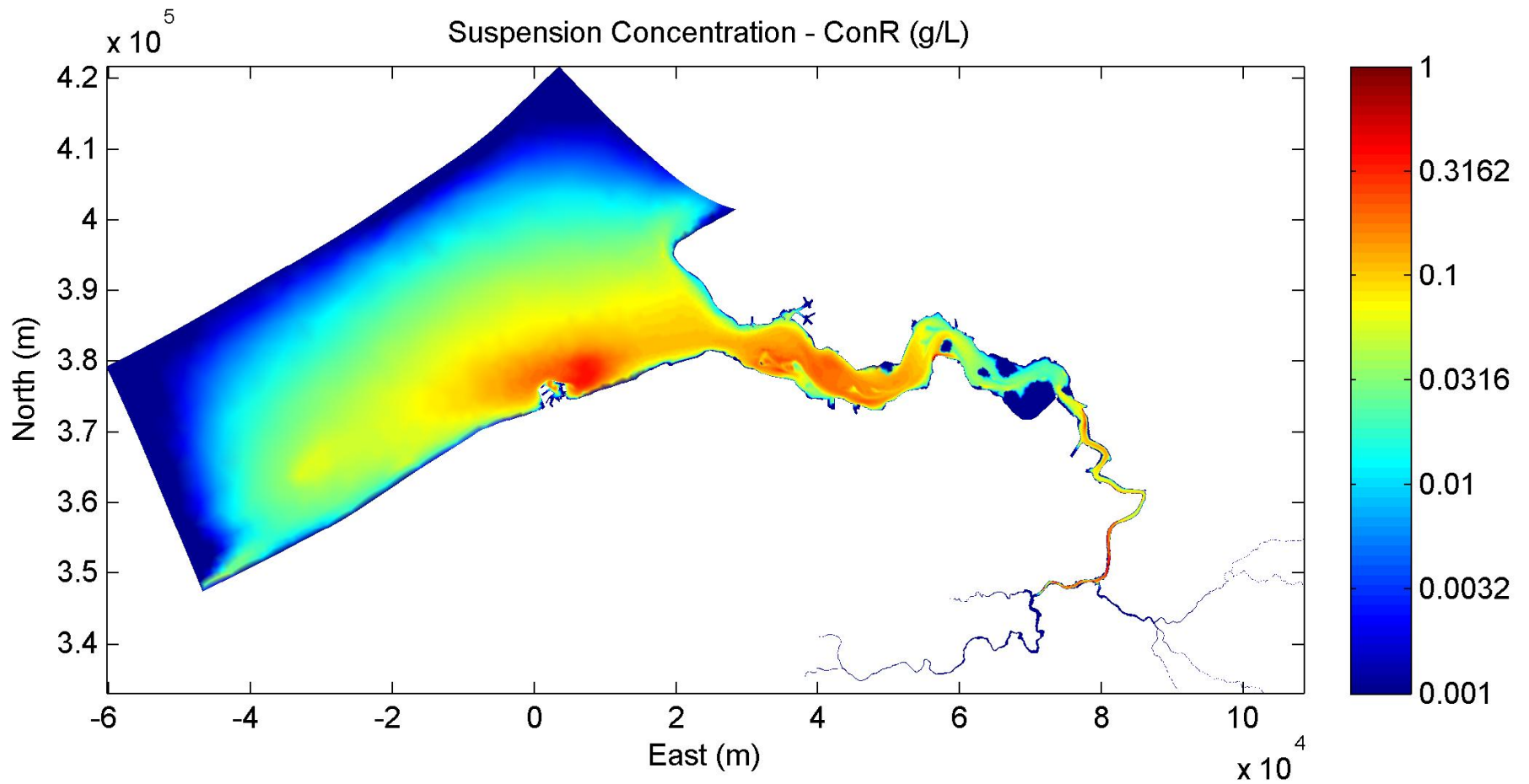


# Model Performance



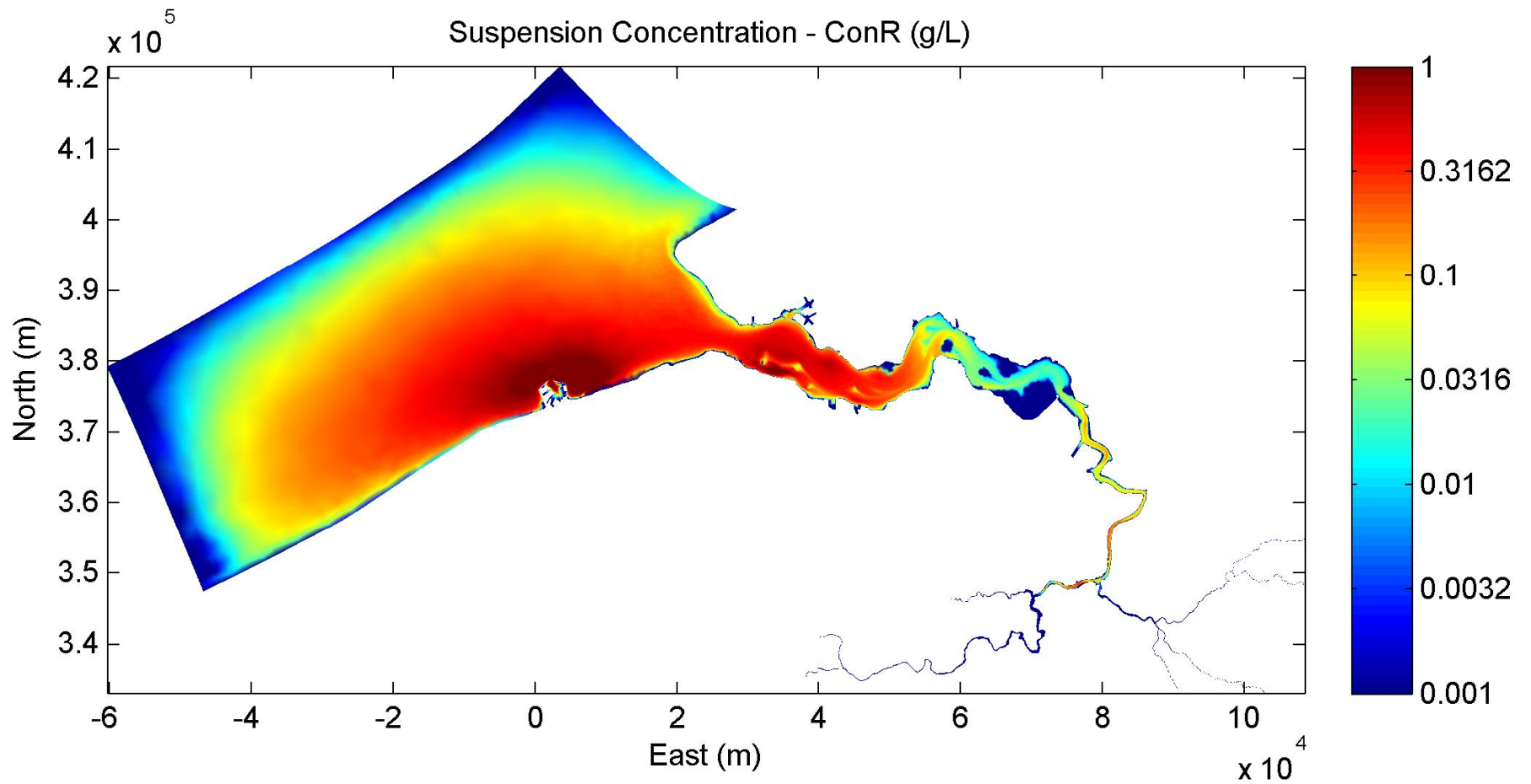
Suspension concentration at step 7000

# Model Performance



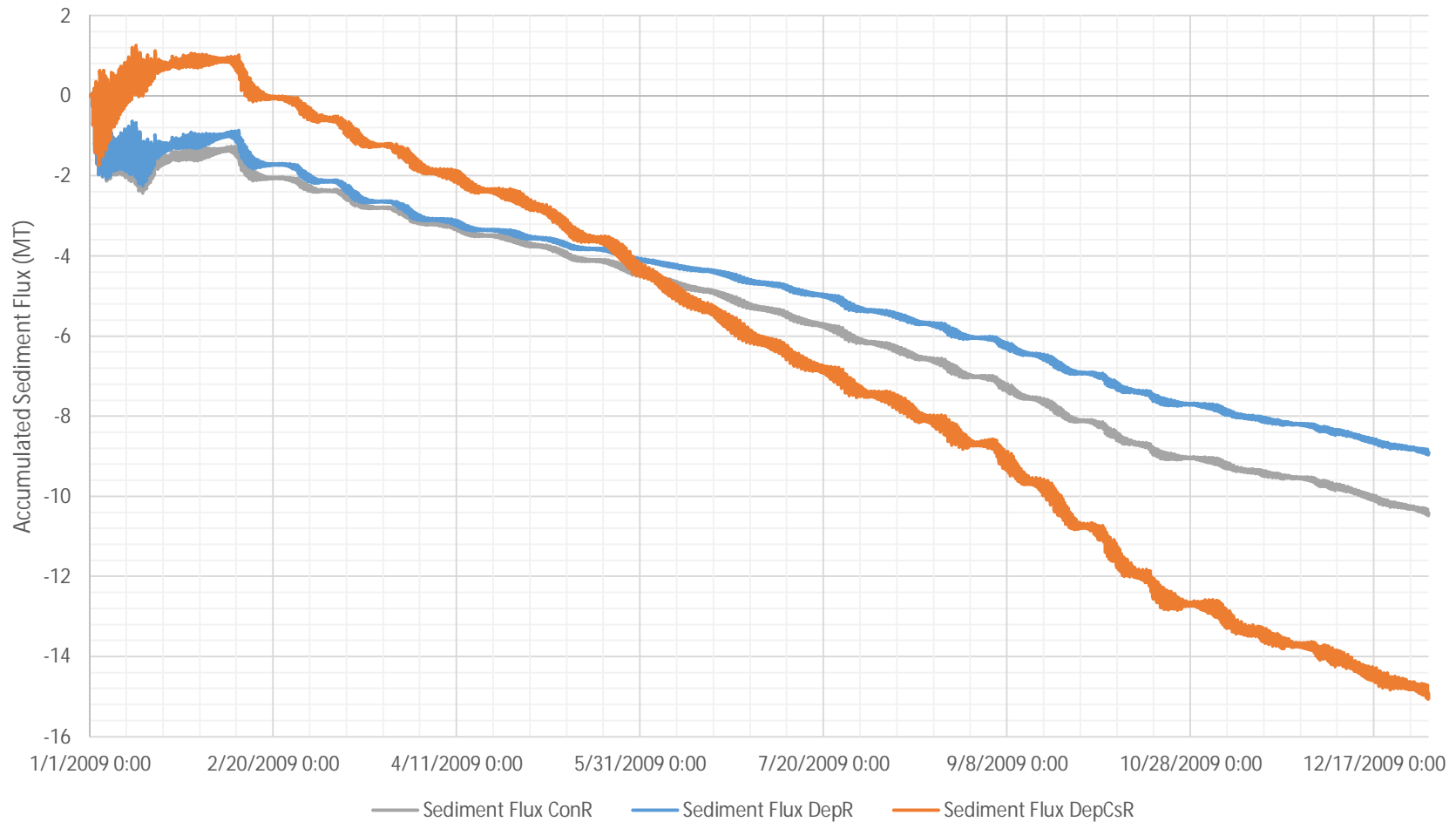
Suspension concentration at step 7000

# Model Performance



Suspension concentration at step 7000

# Model Performance



sediment exporting to the North Sea

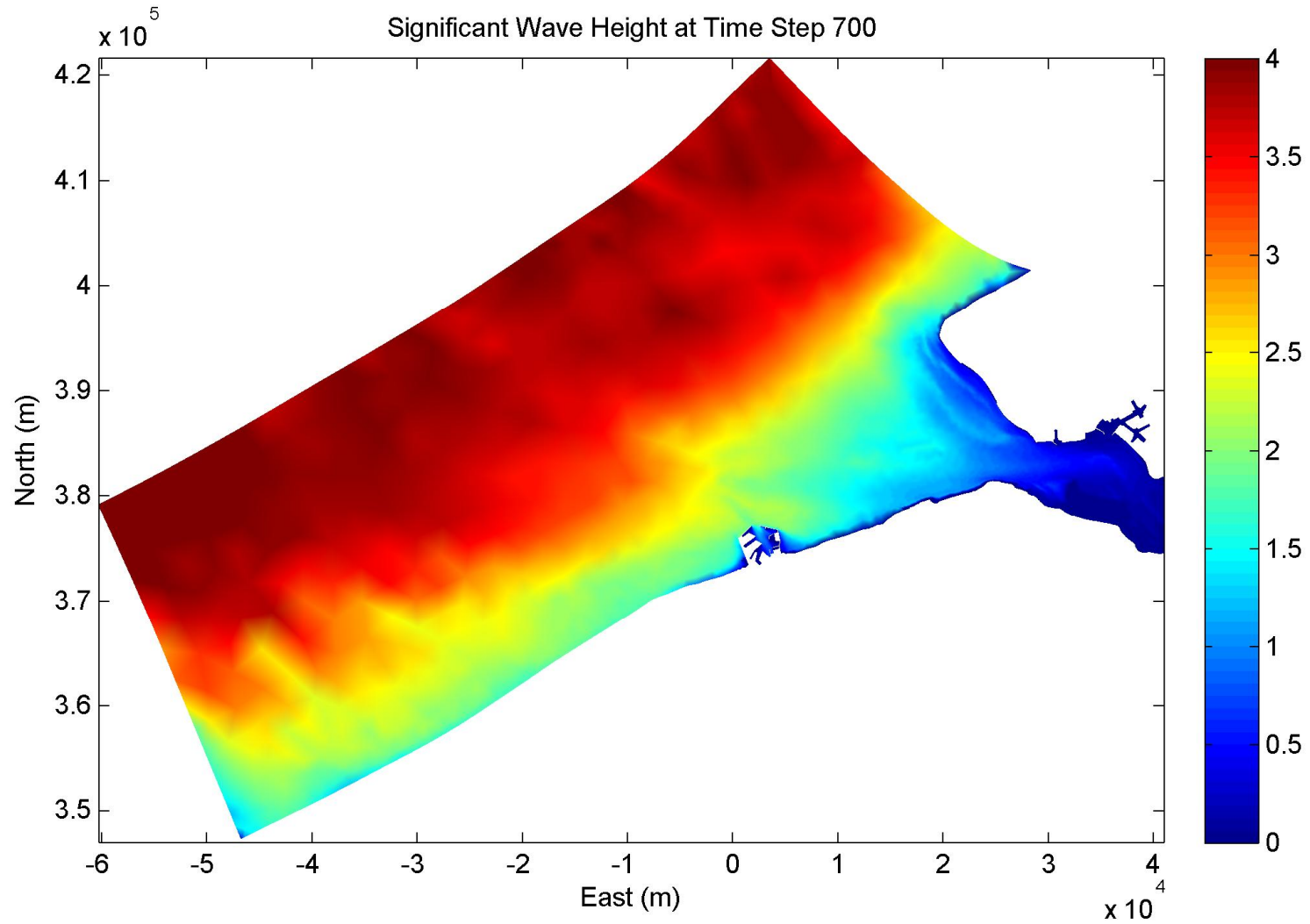
# Costal Waves

- Stationary wind towards the coast
- Boundary significant wave height = 1.5m
- Boundary peak frequency = 0.125

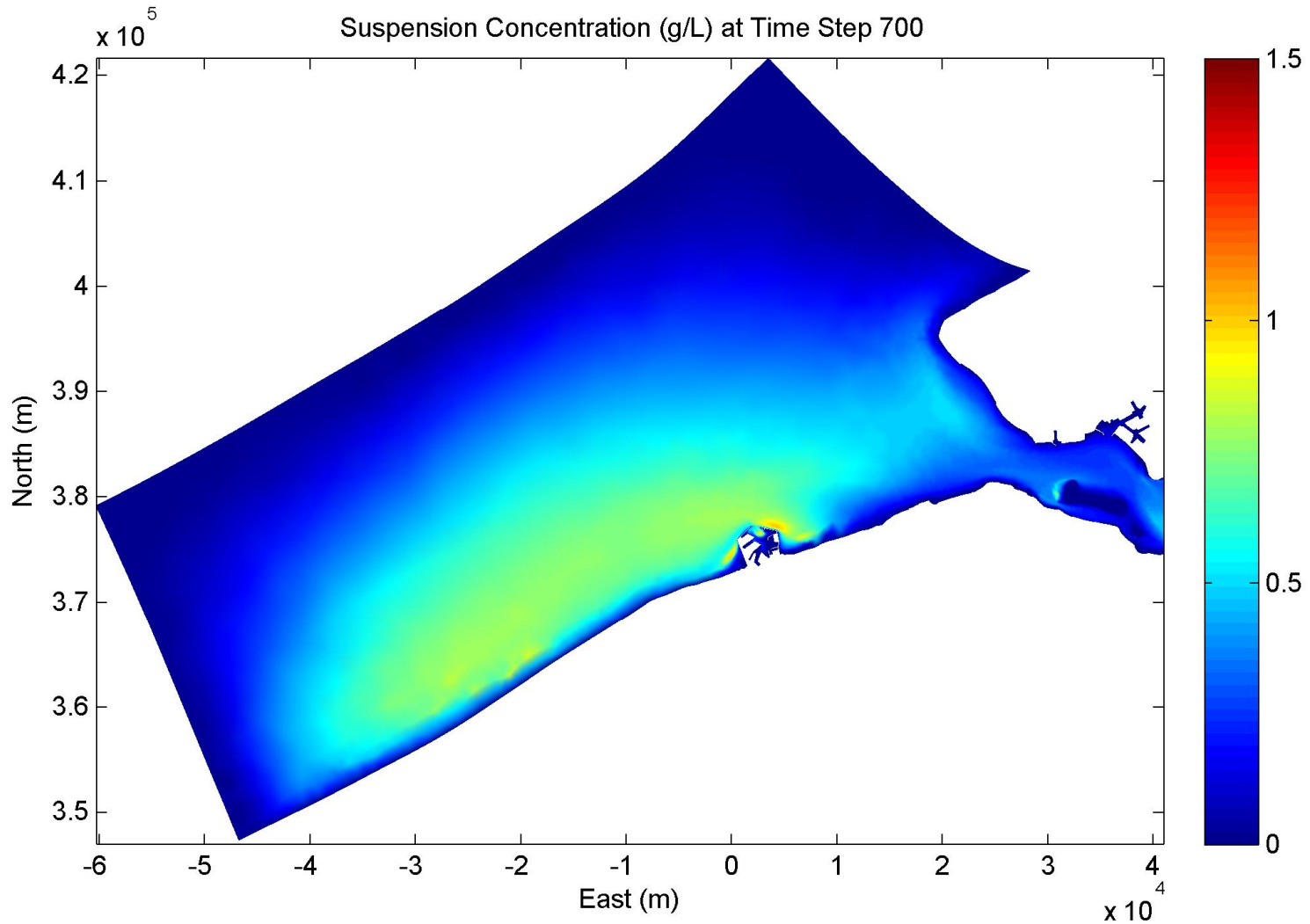


- Three-way coupling
- Wave-current interactions

# Costal Waves

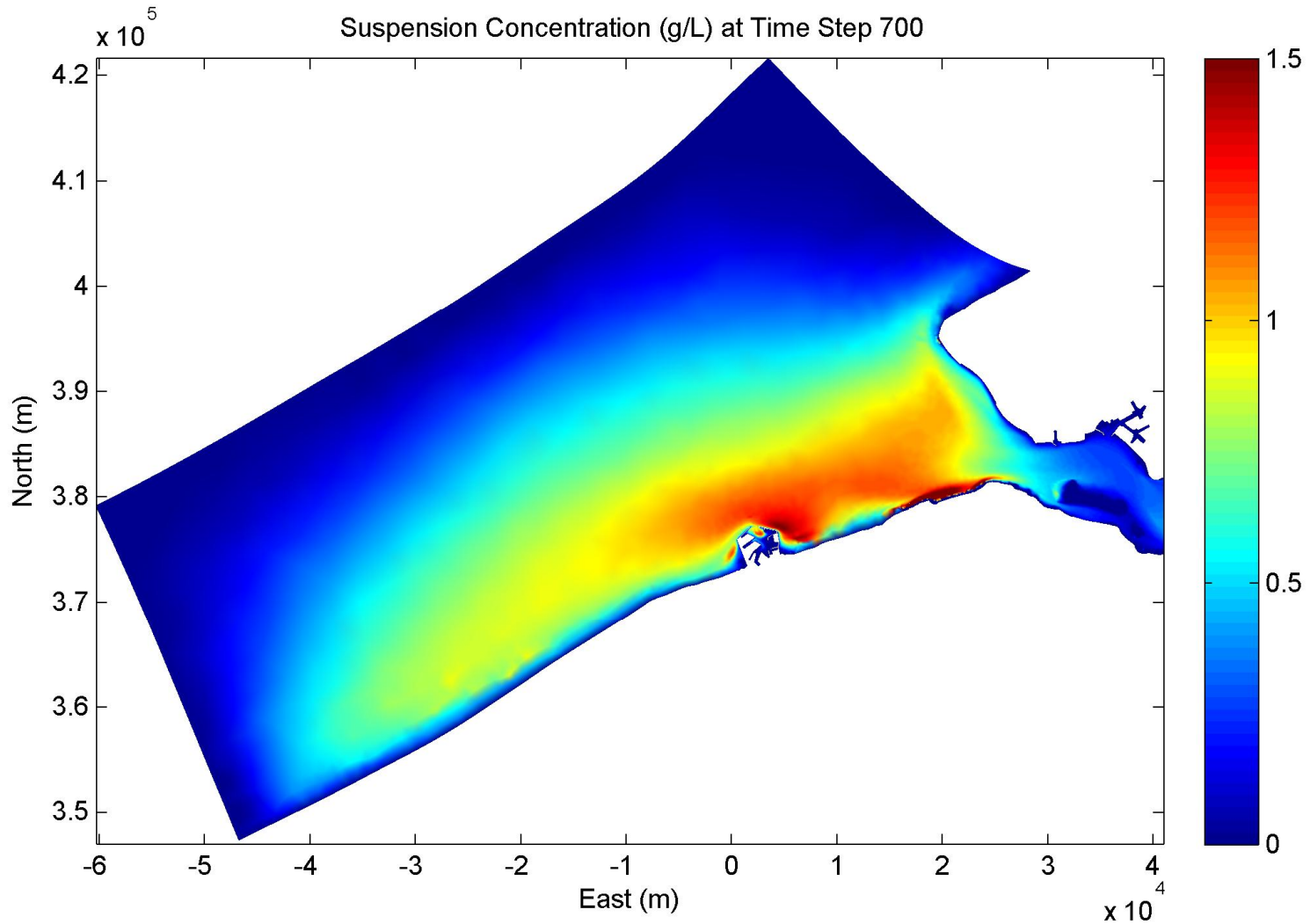


# Costal Waves



(without costal waves)

# Costal Waves



(with costal waves)



Thanks for your  
attention!